

CALIFORNIA DIVISION OF HIGHWAYS
BRIDGE DEPARTMENT COMPUTER SERVICE

COMPOSITE GIRDER CROSS SECTION ANALYSIS - FLANGE DESIGN

INSTRUCTIONS FOR USERS

This service will compute composite girder section properties, stresses in the girder flanges and reinforcing steel and flange sizes required to produce near allowable flange stresses.

DATA PREPARATION (On Form H-BD D 34, Rev. 7-72)

Identification: IDENT and accounting data are standard.
See General Instructions for Users, 1-1.

Use TRIAL and COMMENTS to identify the problem.

Girder Description: SLAB data are needed if the slab is in compression, but not if it is in tension. DIMENSION A is the distance from the top of the deck to the C. G. of the slab. AREA OF EFFECTIVE SLAB is the actual area to be used, not the transformed area.

Give BAR STEEL data if the steel in the slab is to be considered. DIMENSION B is the distance from the top of the deck to the C. G. of the steel in the slab. Show the AREA OF BAR STEEL per girder in square inches.

Some WEB data must be given. DIMENSION Y, the distance from the top of the deck to the top of the web, must be given if SLAB or BAR STEEL data is given. The DEPTH and THICKNESS of the web must always be given.

Various combinations of FLANGE and MOMENT data will produce the following results:

1. Analysis

Section properties and stresses are reported when MOMENTS and all the FLANGE dimensions are given. When MOMENTS are not given, section properties can be obtained for any logical combination of other data.

2. Design

When MOMENTS are given, the design of one or both FLANGES may be left to the computer. Either or both dimensions of a flange may be left blank. The stress in the designed flange(s) will be near allowable. If both dimensions of a flange are given, the stress in that flange is not compared to the allowable stress. When both dimensions of a flange are calculated, it will be approximately 15 times as wide as it is thick. Section properties and stresses are reported for all conditions.

MOMENTS may be applied to the steel girder alone, the 3(n) composite section and/or the 1(n) composite section.

n, the ratio of the modulus of elasticity of steel to that of concrete, is assumed to be 10 if not given.

If the ALLOWABLE STEEL STRESS in the girder flanges is not given, it is assumed to be 20 ksi.

RESULTS

For each problem, the input is reported first, as given or computed. Second, section properties and areas of steel are reported. Last, the stresses in the bar steel and the top and bottom flanges are reported.

The units of the results are:

C/I -- $\text{in/in}^4 \times 12 \times 1000$. If the moment is in ft-kips, then $M \times C/I$ will produce stresses in psi.

Y-BAR -- inches. The distance from the base of the bottom flange to the C.G. of the section.

I -- in^4

Q/I -- in^3/in^4

STRESS -- kips/ in^2

AREAS -- in^2

EXAMPLES

Following are examples intended to illustrate the main features of the program.

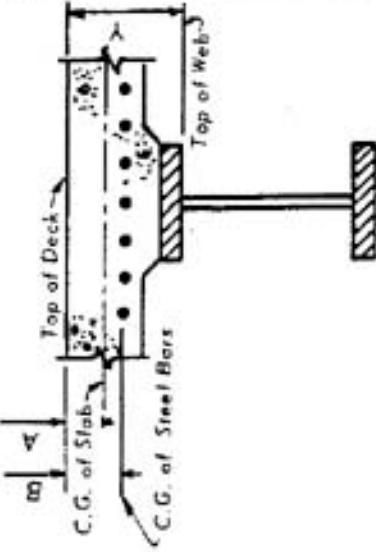
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COMPOSITE GIRDERS

CROSS SECTION ANALYSIS-FLANGE DESIGN

PAGE / OF /
NAME L.I. BIDO
PHONE 5-6518

S/C 1248 209 | S/C 209 |



IDENT	PROB	SOURCE	CHARGE	EXP. AUTH.	SPECIAL DESIGNATION (USE WHEN APPROPRIATE)	
					GEN. LED	SUB-ACT
157	15	15	15	15	15	15
147	0501	1403314033910002				

S/C 1248 209 | S/C 209 |

TRIAL	COMMENTS	DIMENSION A 0.1 IN.	AREA OF SLAB 50.1 IN. EFFECTIONAL	DIMENSION B 0.1 IN.	AREA OF SLAB 50.1 IN. EFFECTIONAL	DIMENSION Y 0.1 SD. IN.	DEPTH 0.1 SD. IN.	THICKNESS 1/16 IN.	WIDTH 1/8 IN.	CURB & RAILING ON 3d CONCRETE FT-K	LIVE LOAD ON 1d CONCRETE FT-K	DEAD LOAD ON STEEL ONLY FT-K	APPLIED MOMENTS IN. STEEL STRESS FT-K	STEEL STRUSS FT-K	ALLOWABLE STRESS FT-K	
0.1	DESIGN, POS.	21	24	28	31	35	18	41	43	45	47	49	51	55	61	65
0.2	SELECT PROPS	511.260	59	125/120	66	520	103.0	1.5								
0.3	DESIGN, NEG		59	125/120	66	520	30									
0.4	ANALYSIS	59	125/120	66	520	1030/4	3.000	300	300	300	300	300	300	300	300	300

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COMPOSITE GIRDERS CROSS SECTION ANALYSIS / FLANGE DESIGN

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TRIAL 01

DESIGN POS

SLAB BAR STEEL
 DIM A= 5.1 DIM B= 5.9 DIM Y = 12.0 WFB
 AREA = 1260. DEPTH = 12.5 DEPTH = 66. WIDTH = 20.
 STEEL GIRDER AREA = 101.9 THICK = 5. THICK = 10. DEAD LOAD = 2875.
 AREA = 20.6 AREA = 25.0 ARFA = 56.3 CURBRAIL = 250.
 ALL=21.

FLANGE					
C/I (PARS)	C/I (TOP FL)	C/I (BOT FL)	Y-RAB	STRESS (BARS)	
DEAD LOAD	6.008	3.275	24.4	89355.	(TOP FL)
CURB & RAIL	2.149	1.824	41.9	179214.	9.4
LIVE LOAD	1.034	0.790	2.679	0.00995	0.7
				239055.	0.01235
					4.1
					3.2
					10.7
				TOTAL	4.7
					20.9
					20.8

TRIAL 02

SECT PROPS

SLAB BAR STEEL
 DIM A= 5.1 DIM B= 5.9 DIM Y = 12.0 WFB
 AREA = 1260. AREA = 12.5 DEPTH = 66. WIDTH = 20.
 STEEL GIRDER AREA = 101.9 THICK = 5. THICK = 10. DEAD LOAD = 2875.
 AREA = 20.6 AREA = 25.0 AREA = 56.3 CURBRAIL = 250.
 ALL=21.

FLANGE					
C/I (PARS)	C/I (TOP FL)	C/I (BOT FL)	Y-RAB	STRESS (BARS)	
DEAD LOAD	6.008	3.275	24.4	89355.	(TOP FL)
CURB & RAIL	2.149	1.824	41.9	179214.	9.4
LIVE LOAD	1.034	0.790	2.679	0.00995	0.7
				239055.	0.01235
					4.1
					3.2
					10.7
				TOTAL	4.7
					20.9
					20.8

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COMPOSITE GIRDERS CROSS SECTION ANALYSIS / FLANGE DESIGN

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TRIAL 03

DESIGN NEG

SLAP BAR STEEL
 DIM B = 5.9 DIM Y = 12.0 WIDTH = 20.
 AREA = 0. DEPTH = 12.5 THICK = 18.
 AREA = 0. CURBRAIL = 14.
 THICK = 5.
 STEEL GIRDERS AREA = 110.1 AREA = 20.6 AREA = 52.5

	FLANGE			BOTTOM			MOMENTS		
	C/I (BARS)	C/I (TOP FL)	C/I (BOT FL)	WEB	TOP	BOTTOM	DEAD LOAD	CURBRAIL	N = 10. ALL = 21.
DEAD LOAD	3.741	3.279	32.7	119668.					
CURB C RAIL	2.864	2.167	36.6	138814.	0.00335		11.2	9.8	
LIVE LOAD	3.217	2.884	3.167	36.6	138814.	0.00335	1.0	0.9	1.0
							9.7	8.7	9.5
					TOTAL	10.6	20.7	20.3	

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TRIAL 04.

ANALYSIS

SLAB BAR STEEL
 DIM B = 5.9 DIM Y = 12.0 WIDTH = 20.
 AREA = 0. DEPTH = 12.5 THICK = 18.
 AREA = 0. CURBRAIL = 14.
 THICK = 5.
 STEEL GIRDERS AREA = 110.1 AREA = 20.6 AREA = 52.5

	FLANGE			BOTTOM			MOMENTS		
	C/I (BARS)	C/I (TOP FL)	C/I (BOT FL)	WEB	TOP	BOTTOM	DEAD LOAD	CURBRAIL	N = 10. ALL = 20.
DEAD LOAD	3.741	3.279	32.7	119668.					
CURB C RAIL	2.864	2.167	36.6	138814.	0.00335		11.2	9.8	
LIVE LOAD	3.217	2.884	3.167	36.6	138814.	0.00335	1.0	0.9	1.0
							9.7	8.7	9.5
					TOTAL	10.6	20.7	20.3	

BATCH CHARGE UNITS = 4 APPROXIMATE MACHINE TIME 0.8 SEC'S APPROXIMATE COST \$ 5.80